

ANALYSIS OF CONTINUOUS BEAM STRENGTHENED BY CARBON FIBRE REINFORCED POLYMER (CFRP) USING ANSYS SOFTWARE

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Abstract:

Structures made of reinforced concrete have a shorter service life every day. This is a result of the reinforced structural elements—beams, columns, walls, floors, etc.—degrading. Numerous circumstances, such as heavy loads, fires, earthquakes, mistakes in the design, chemical attacks, etc., might cause these components to be damaged. Retrofitting procedures can be used to strengthen these structural elements.

Retrofitting is already spreading like wildfire over the globe as many historically significant public and private structures get really old and weaken over time. One of the best ways to make an insufficient building safe against future earthquakes or other natural pressures is to retrofit it. Retrofitting is the process of incorporating new elements into existing constructions such as bridges, historic buildings, and so forth. A retrofitted building is less likely to sustain damage from seismic activity in the near future. It seeks to reinforce a structure in order to meet the demands of the current seismic design rules. Retrofit goes beyond simple repair or even rehabilitation in this regard. It involves modifying existing structures to increase their resistance to seismic action, ground motion, and soil collapse brought on by earthquakes or other natural disasters like tornadoes, cyclones, and winds with high velocity generated by thunderstorms, snowfall, hailstorms, etc. Structures gradually lose their strength over time, although some are crucial from a public and social perspective. Retrofitting extends the structure's overall strength, resistance, and longevity. RC beam retrofitted with various thermoplastic polymer composite sheets was subjected to a finite element study using the Ansys V20 programme. Utilizing Ansys software, RC beams with various thermoplastic sheets were modelled. The bottom, top, and both sides were composed of bonding. The findings of the comparison between the reinforced beam and the aforementioned retrofitted beam's performances were presented in this project.

The project's goals are to hand design a G+3 structure before using ANSYS SOFTWARE to model the external continuous beam. After modelling, we use ANSYS SOFTWARE to statically analyse this continuous beam, and then we compute the results by both mannual and software. We recognise that there is little difference between the results obtained from Case 1 (solved in Ansys) and the calculations performed manually. The beam without wrapping experienced the largest deviation, measuring 6.8mm, while the wrapped beam only saw a 1.02mm deflection under controlled conditions. The best results are obtained while wrapping on CFRP sheet with a 450 degree orientation. The maximum shear strength of a wrapped beam is 90.951 KN, while the maximum shear strength of a beam without wrapping is 68.071 KN. The greater the maximum shear force that the beam can withstand, the more durable the beam is. The maximum BMD provided by the wrapped beam is 62.1*106 KNm, whereas the unwrapped beam provides BMD of 36.3*106 KNm. The percentage decrease in the deflection between both the beams can be seen as 15%.

Keywords: CFRP, RC , retrofit , compressive strength, BMD, SFD

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1. INTRODUCTION

Reinforced concrete structures often have to face modification and improvement of their performance during their service life. The main contributing factors are, change in their usage, new design standards, corrosion in steel caused by exposure to an environment and accidental events such as earthquakes. In such circumstances there are two possible solutions, replacement or retrofitting. Full structure replacement might have determinate disadvantages such as high costs for material and labour, a stronger environmental impact and inconvenience to public. One of several reasons that cause the collapse of a multi-story building or bridge structure is the failure of the supporting members to withstand the earthquake loading. The failure of these members is mostly due to the lack of shear-resisting capacity and insufficient ductility provided by little amount of transverse steel. It is well known that the ductility of a reinforced concrete plays a very important role in preventing such a failure. That is why the study on the ductility of a reinforced concrete has been developing in the last two decades in many countries worldwide. One of the effective ways to improve the ductility of a member is by introducing sufficient transverse steel as confining steel for concrete core in a column. This effort is primarily intended to delay the sudden collapse of a structure and force it further to fail in a ductile manner.

When possible, it is often better to repair or upgrade the structure by retrofitting. The development of strong epoxy has led to a technique which has great potential in the field of upgrading structures. Basically the technique involves gluing steel plates or fibre-reinforced `with the concrete and help to carry the loads. Fibre-reinforced polymer (FRP) systems have been used to strengthen and retrofit existing concrete structures around the world since the mid-1980s. The number of projects using FRP systems worldwide has increased, from a few 20 years ago to several thousand today. Structural elements strengthened with FRP systems include beams, slabs, columns, walls, joints/connections, etc. FRP systems have also been used to strengthen masonry, timber, steel, and cast-iron structures. The behaviour of FRP strengthened reinforced concrete (RC) members is more complex than unstrengthen RC members and therefore, a clear understanding of their behaviour under different types of loading is important. The initial development of FRP systems for the retrofit of concrete structures occurred in the 1980s in both Europe and Japan.

2. OBJECTIVES

- 1. To study the behavior of reinforced concrete structure.
- 2. To model beam in ANSYS Software
- 3. To statically analyze this beam in ANSYS Software

3.METHODOGY

Methodology is 'a contextual framework' for research, a coherent and logical scheme based on views, beliefs, and values that guides the choices researchers, or other users, make. It comprises the theoretical analysis of the body of methods and principles associated with a branch of knowledge such that the methodologies employed from differing disciplines vary depending on their historical development. This creates a continuum of methodologies that stretch across competing understandings of how knowledge and reality are best understood. This situates methodologies within overarching philosophies and approaches. Overall, a methodology does not set out to provide solutions therefore, not the same as a method. Instead, a methodology offers a theoretical perspective for understanding which method, set of methods, or best practices can be applied to the research question/questions at hand.



Figure 3.1Structural Plan of G+3 Builiding

DIMENSION OF BEAM

Beam Length 15880 mm depth, 266 mm and 230 mm



Figure 3.2 Dimension of selected Beam

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PROPERTIES OF MATERIALS

Material	Descriptions	Specification
	Weave style	Hexcel woven (T- 700)
	Areal weight of fabric (g/m^2)	98-670
	Standard width (mm)	1000
CFRP	Dry fabric thickness(mm)	0.35
	Tensile strength (Mpa)	3500
	Tensile modulus (Gpa)	228
	Elongation percentage (%)	4.761
	Grade of steel (N/mm ²)	415
Steel	Tensile Stress of Steel (MPa)	485
	Elongation of Steel (%)	20
	Grade of Concrete (MPa)	20
	Compressive Strenght (N/mm ²)	20
Concrete	Density (Kg/m ³)	2400
	Proportion (Ratio)	1:1.5:3
	Poission's Ratio	0.2
	Modular Ratio	13.33
	Tensile Strength (MPa)	25
Epoxy Resin (Sikadur C31)	Modulus of Elasticity (MPa)	5200
	Flexural Strenght (MPa)	6900
	Tensile Strength (MPa)	45
Epoxy Resin (Sikadur C300)	Modulus of Elasticity (MPa)	3500

Table 3.1 Properties of Materials.

Flexural 3000 Strenght (MPa)

Modelling of beam

A Continuous Beam with dimensions shown in Fig. 4 has been drawn in the present case. ANSYS V20 has been used to model the beam and the analysis part has also been conducted. The combination of loads (UDL) of 32.65 KN/m, 29.11 KN/m, 34.37 KN/m, 29.11 KN/m & 42.04 KN/m has been adopted to see the effect of the load on the Continuous Beam. The total deflection, SFD, BMD and Von-mises stress results have been shown.

4. RESULT & DISCUSSION

Finite Element Method (FEM) method has been used in order to obtain the analytical solution for continuous beam with/without CFRP. In ANSYS, model analysis is used to determine its deformation in deflection with CFRP materials. We found that the beam with CFRP wrap is more durable than normal beam. The results of analysis done in ANSYS is quite more appealing than manually done analysis. In ANSYS, color-coding is used for showing the nodes/elements under stresses. Analytical and FEM results are observed. Analytical predictions are validated with the experimental results taken from literature and a good match was found. Predicted and experimental results for overall deformation are compared for both the specimens

CASE- 1: Here, the beam shown is the beam without CFRP-

Model by using ANSYS Software-



Figure 4.1 Deformed shape of our Continuous

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Beam after applying loads.



Figure 4.1 Isometric view of our deformed Continuous Beam.



Figure 4.2 Total deformation of the Continuous Beam.



Figure 4.3 Bending Moment Diagram.



Figure 4.4 Shear Force Diagram.

CASE- 2: Here,	, the beam	shown is	s bonded	with th	ne
CFRP Composit	e material-	-			



Figure 4.5 Deformation of continuous beam with CFRP.



Figure 4.6 Deformed beam bonded by CFRP

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under loads.



Figure 4.7 Bending Moment Diagram.



Figure 4.9 Shear Force Diagram.

A comparison can be seen in the following table with the total load (UDL) acting on Continuous Beam = 167.28 KN.



Figure 4.8 Stress Distribution.

Type of Beam Solved	Total Deformation (mm)	BMD (KNm)	SFD (KN)	# - Solv ed in Ans ys Soft
Continuous Beam Solved Manually (Without Wrapping)	6.35	35.04*10 ⁶	66.77	ware
Case-1 [#] (Without Wrapping)	6.8	36.3*10 ⁶	68.071	
Case-2 [#] (With Wrapping)	1.02	62.1*10 ⁶	90.951	

Table 4.1 Deformation and Stresses.

5. Conclusion

From the study it is concluded that many existing structures that were built according to past design codes and standards are often found vulnerable to earthquake damage. Due to inadequate detailing, under estimated earthquake loads or material deterioration by time, the high cost of new construction and historical importance of older buildings has led building owners to renovate/retrofit Analysis of continuous beam strengthened by carbon finre reinforced polymer (CFRP) using ANSYS software

the structures rather than replace the existing structures. In this, the Continuous Concrete Beam has been solved in the present problem. FEM (Finite Element Method) software i.e. ANSYS, help in solving the beam problem. The total deflection, SFD, BMD and Von-mises stress results have been plotted.

By studying, we came to the conclusion, that the statistical behaviour cannot be determined in manual analysis, but in Ansys the simulation of the member can be statistically analysed under any loading. The time required for solving the beam is minimized. In manually calculations there are many chances of error & so the Ansys can be used. Due to the complication of the problem, one may find difficult to design and solve the problem, but in Ansys it can be handled smoothly. In manual analysis the minute nodal displacement is same due to symmetry but in Ansys the nodal displacement may vary a little due to the support condition.

Following are the conclusions derived from the studies conducted as a part of this study:

- Some of the concrete elements such as beams may be deficient in Flexural and is in need of strengthening.
- The external wrapping with CFRP sheets can increase the flexural capacity of RC beams.
- CFRP is promising material for strengthening of beams under any loading condition.
- We came to the point that the calculation done manually and the model solved in Case-1 have nearly same values of deflection, i.e. 6.8mm. The major changes can be seen after the wrapping of CFRP sheets over the beam (Case-2).
- The maximum deflection was found in the beam without wrapping (Case-1) having the deflection of 6.8mm. And that to the beam with wrapping (Case-2) gives deflection of 1.02mm which can be in controlled condition.
- Wrapping on CFRP sheet with the orientation of 45[°] gives the best results.
- Maximum shear strength of the beam with wrapping (Case-2) is 90.951 KN. And that to of beam without wrapping (Case-1) is 68.071 KN. The maximum shear force taken by the beam, the more the sustainable the beam is.

- Maximum BMD offered by the Beam with wrapping (Case-2) is 62.1*10⁶ KNm, whereas, the without wrapping (Case-1) gives BMD of 36.3*10⁶ KNm.
- We can conclude that, 'as we increase the elasticity of member it results the more durability of the member'.
- Percentage decrease in the deflection between Case-1 & Case-2 of the beams can be seen as 15%.

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